

Using JULES to simulate infiltration and surface runoff in situations of intense rainfall

JULES Conference
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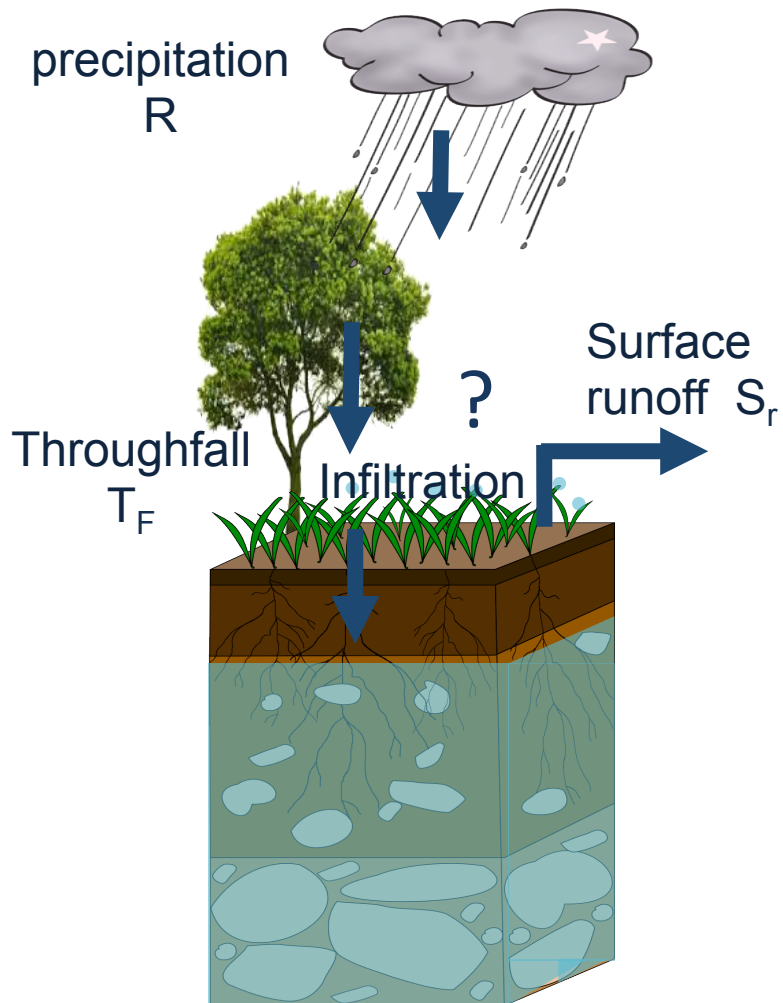
[2] Department of Meteorology, University of Reading



Overview of talk:

1. Infiltration theory
2. Methods:
 1. Methods of infiltration used in Land surface models
 2. New scheme of infiltration in JULES
3. Results: Comparison of observed and modelled river flow for a UK catchment (Ure)
4. Conclusions and Outlooks

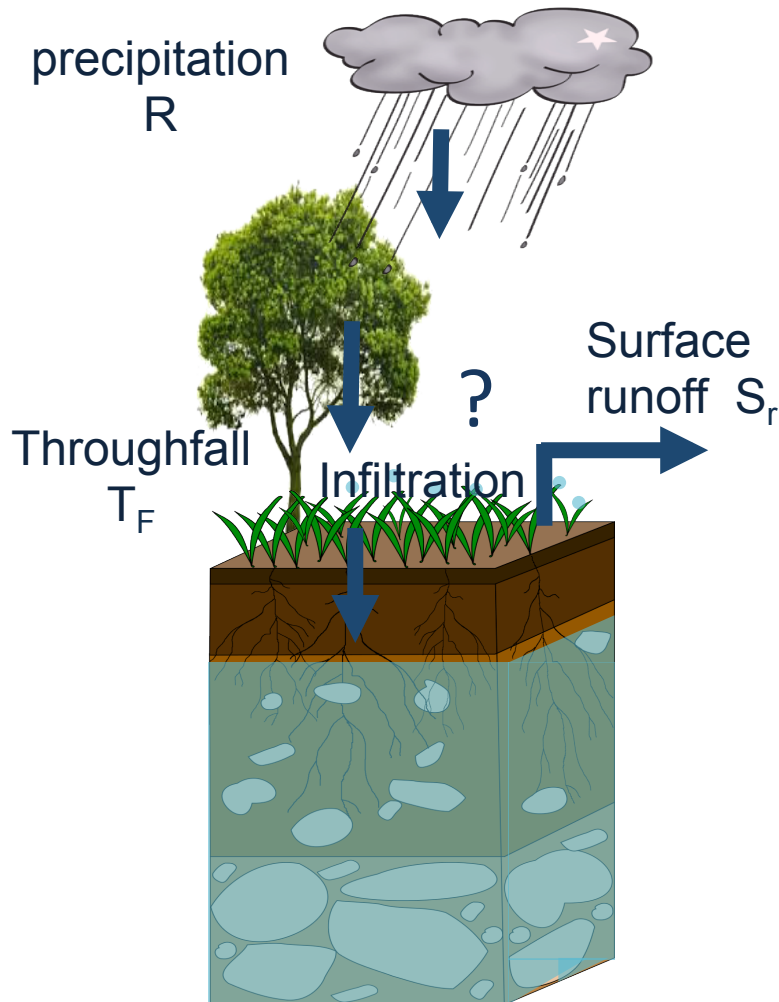
Schematic of infiltration processes:



Actual infiltration depends on value of T_F
and maximum infiltration I_{max}

If $T_F > I_{max}$: Surface runoff = $T_F - I_{max}$
If $T_F < I_{max}$: Infiltration = T_F

Schematic of infiltration processes:



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Standard version of JULES: $I_{max} = \beta K_{sat}$

$20 \text{ mm/d} < K_{sat} = f(\text{soil}) < 1200 \text{ mm/d}$

PFT Number	default value
Broadleaf tree	4.00
Needleleaf tree	4.00
C3 grass	2.00
C4 grass	2.00
Shrubs	2.00
Urban	0.10
Open water	0.00
Bare soil	0.50
Ice	0.00

Infiltration scheme used in Land Surface models:

Model	Institution	Reference	Maximum infiltration method	Actual infiltration method
JULES	Met Office	(Best et al., 2011)	Fixed I_{max} rate	SWB
VIC	Princeton Uni.	(Gao et al., 2010) (Liang et al., 1996)	VIC scheme	SWB
ISBA	Meteo-France	(Decharme and Douville, 2006) (Noilhan and Mahfouf, 1996)	VIC scheme	SWB
ORCHIDEE	IPSL	(Krinner et al., 2005)	VIC $I_{max}=f(\theta)$	probability distrib.
CLM	NCAR	(Oleson et al., 2010)	VIC $I_{max}=f(\text{texture}, \theta)$	SWB
HTESSEL	ECMWF	(Balsamo et al., 2009)	VIC $I_{max}=f(\theta, \text{orog.})$	SWB
NOAH	NCEP	(Schaake et al., 1996)	VIC $I_{max}=f(\theta, K_{sat})$	SWB
CLASS	Canada	(Verseghy, 1991)	Green-Ampt	Green-Ampt $I=f(K, \Psi)$
CABLE	Australia	(Haverd and Cuntz, 2010)	No I_{max}	SWB
MATSIRO	Japan	(Takata et al., 2003)	No I_{max}	SWB
G2G	CEH	(Bell et al., 2007, 2009)	VIC $I_{max}=f(\theta)$	probability distrib.

Adaptation from the work of H.Ashton

VIC: Variable Infiltration Capacity

SWB: Surface Water Balance

$$I = \sum_{i=1}^n PFT (T_{fall} + M - S_{runoff})$$

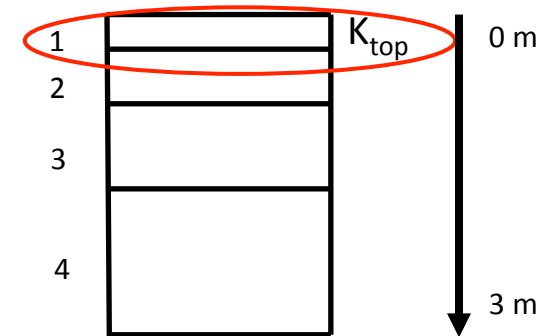
Variable maximum infiltration schemes:

- Standard Scheme

$$I_{max} = \beta K_{sat}$$

- New scheme of infiltration

$$I_{max} = \beta K_{top}$$



Comparison of river flow in a small catchment:

- Standard Scheme $I_{max} = \beta K_{sat}$ (CTL)
- New scheme of infiltration $I_{max} = \beta K_{top}$ (βK)
- Scheme PDM activated (PDM)
- Scheme PDM deactivated (NO PDM)
- Observation (National River Flow Archive)

PDM scheme :

- Calculation of the fraction of the grid which is saturated F_{sat}
- Generate surface runoff from saturation excess

$$F_{sat} = 1 - \left(\frac{S - S_0}{S_{max} - S_0} \right)^{\frac{b}{b+1}}$$

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Evaluation of the model : Kling-Gupta Efficiency

$$KGE = 1 - \sqrt{\underbrace{\left(\frac{Cov_{sim,obs}}{\sigma_{sim}\sigma_{obs}} - 1\right)^2}_{\rho} + \underbrace{\left(\frac{\sigma_{sim}}{\sigma_{obs}} - 1\right)^2}_{a} + \underbrace{\left(\frac{\mu_{sim}}{\mu_{obs}} - 1\right)^2}_{b}}$$

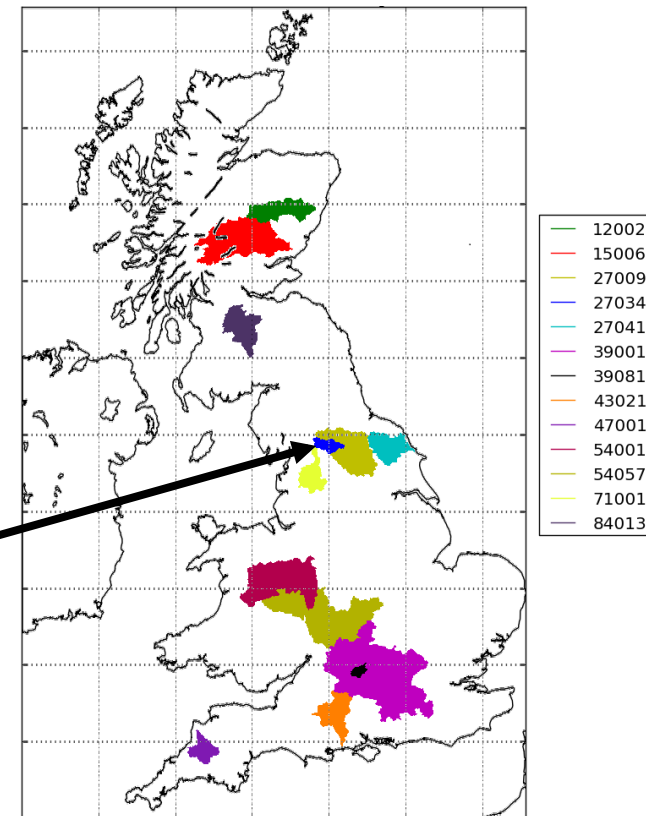
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Ure catchment area: 510 km²
10 years period: 1991-2000

meteorological forcing used: CHESS (CEH)

- 1 km² spatial resolution
- daily precipitation
- using RFM for each simulation

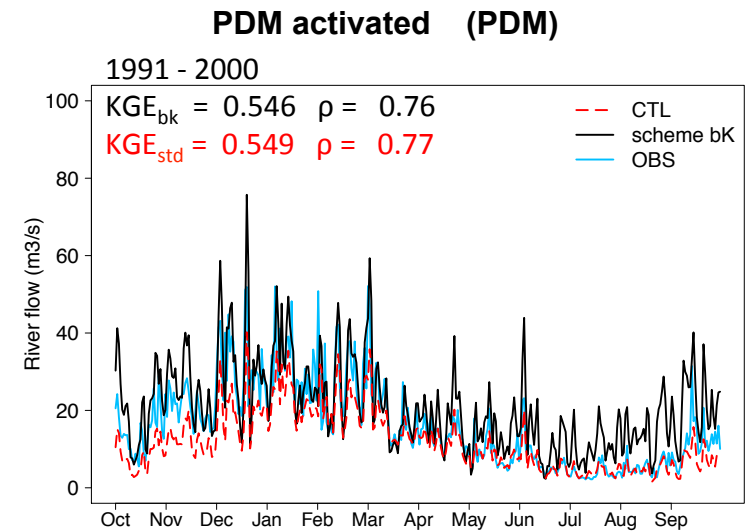
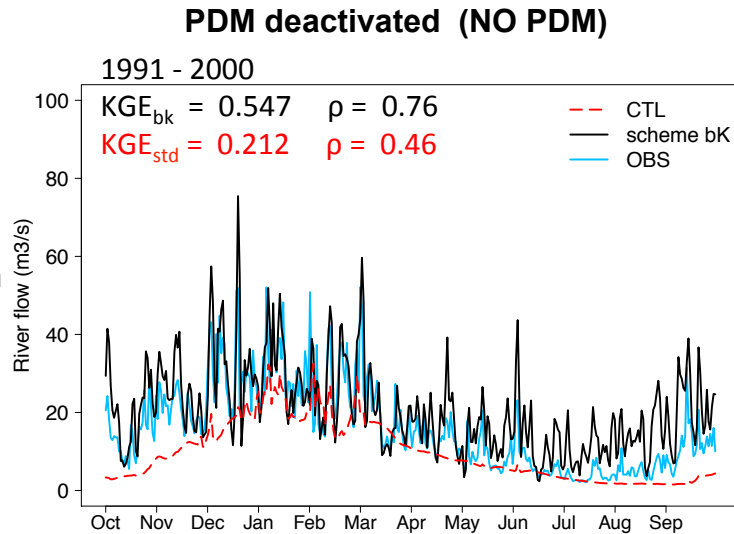


Res. Note UKEP A.Martinez

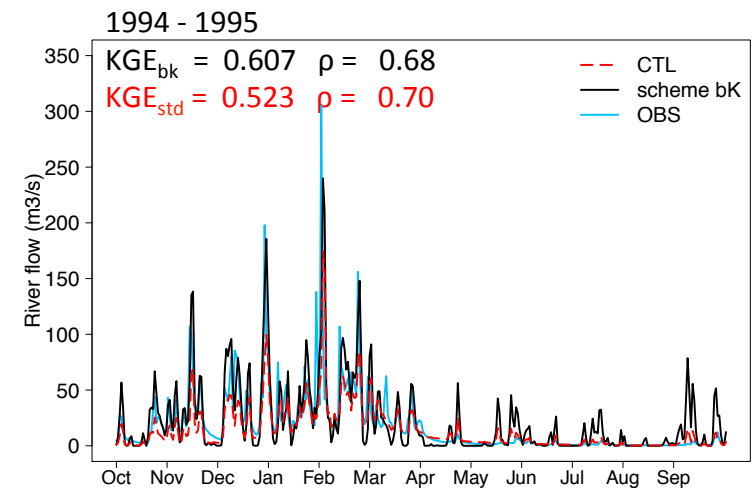
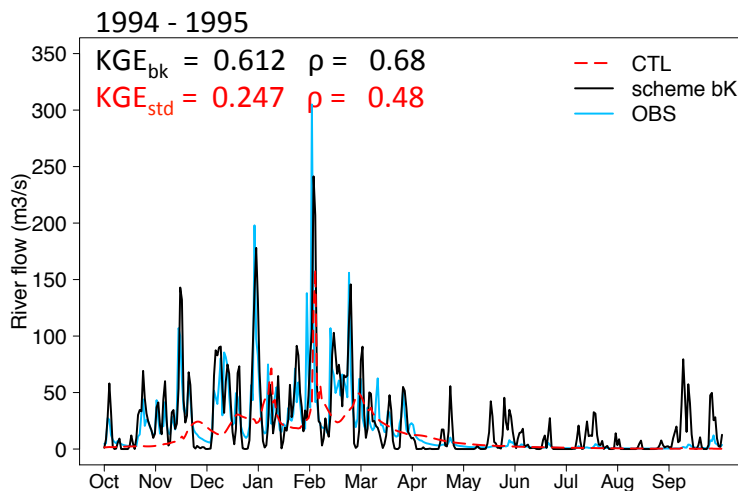
New scheme (scheme bK)
Standard scheme (CTL)

Comparison of river flow in a small catchment:

Daily annual mean
1991 - 2000



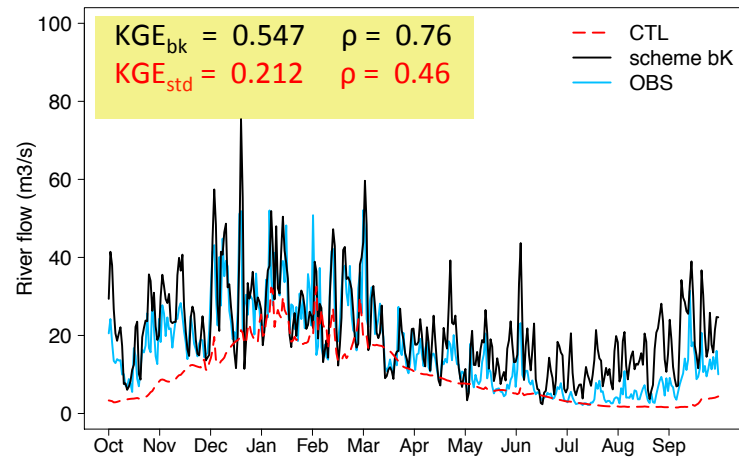
Oct. 1994–1995
Peak river flow
period



New scheme (scheme bK)
Standard scheme (CTL)

Comparison of river flow in a small catchment:

PDM deactivated (NO PDM)



1991 - 2000

$\rho = 0.76$ $\rho = 0.46$

$b = 1.28$ $b = 0.63$

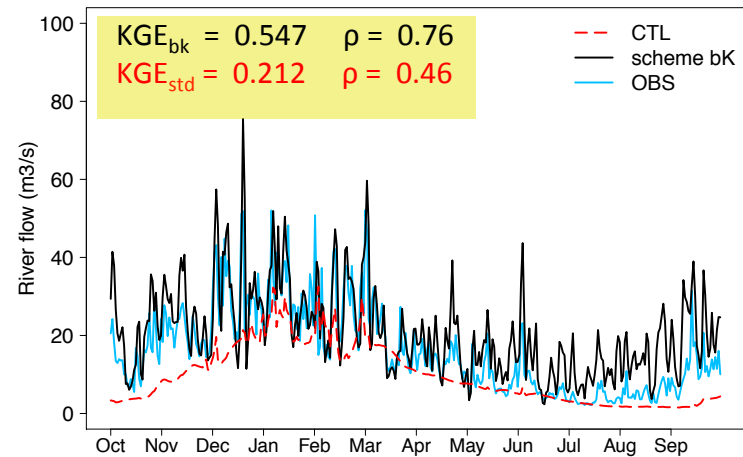
$a = 1.26$ $a = 0.55$

$$KGE = 1 - \sqrt{\underbrace{\left(\frac{Cov_{sim,obs}}{\sigma_{sim}\sigma_{obs}} - 1\right)^2}_{\rho} + \underbrace{\left(\frac{\sigma_{sim}}{\sigma_{obs}} - 1\right)^2}_{a} + \underbrace{\left(\frac{\mu_{sim}}{\mu_{obs}} - 1\right)^2}_{b}}$$

New scheme (scheme bK)
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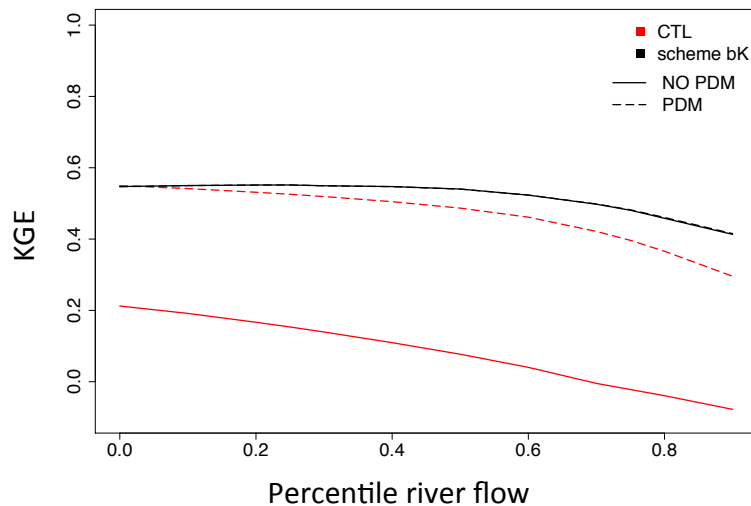
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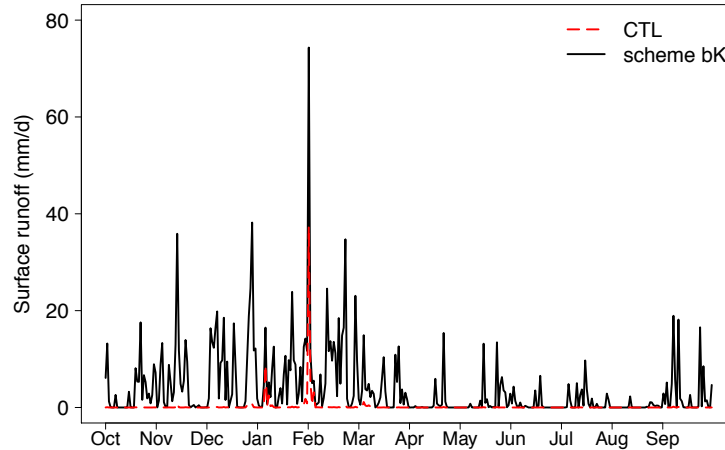
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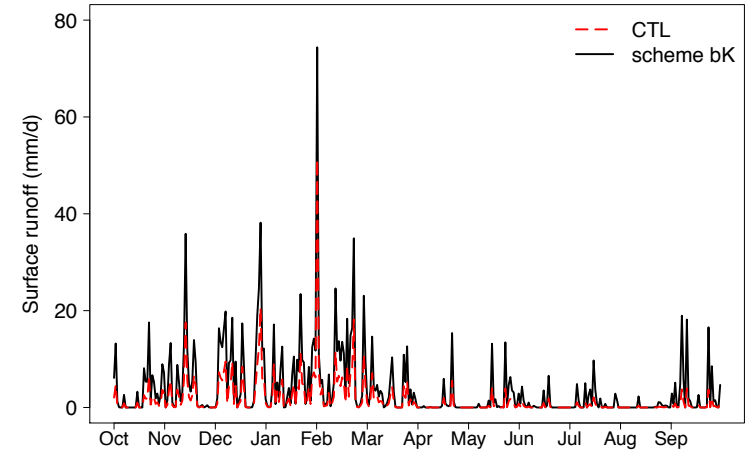
New scheme (scheme bK)
Standard scheme (CTL)

Comparison of surface runoff in a small catchment:

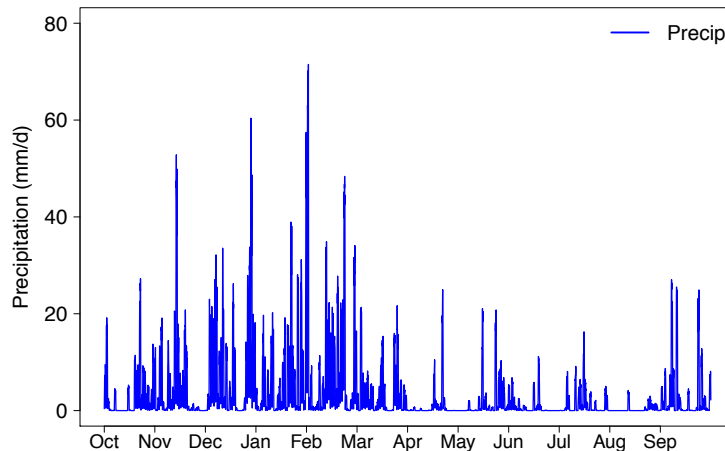
PDM deactivated (NO PDM)



PDM activated (PDM)



**Surface runoff
Oct. 1994–1995
Peak river flow
period**



Srunoff:

$$\rho (\text{bk NO PDM, bK PDM}) = 0.9999$$

$$\rho (\text{bk PDM, CTL PDM}) = 0.967$$

Riverflow:

$$\rho (\text{bk NO PDM, bK PDM}) = 0.999$$

$$\rho (\text{bk PDM, CTL PDM}) = 0.92$$

Conclusions:

1. Land surface models used mainly : max VIC and infiltration is based on SWB
2. The scheme $I_{\max} = \beta K$:
 - i. Enhance an increase of the surface runoff
 - ii. Improve the river flow in a small catchment when high precipitation occurs
 - iii. Overestimate the mean river flow (parameter b) and the variability (parameter a)

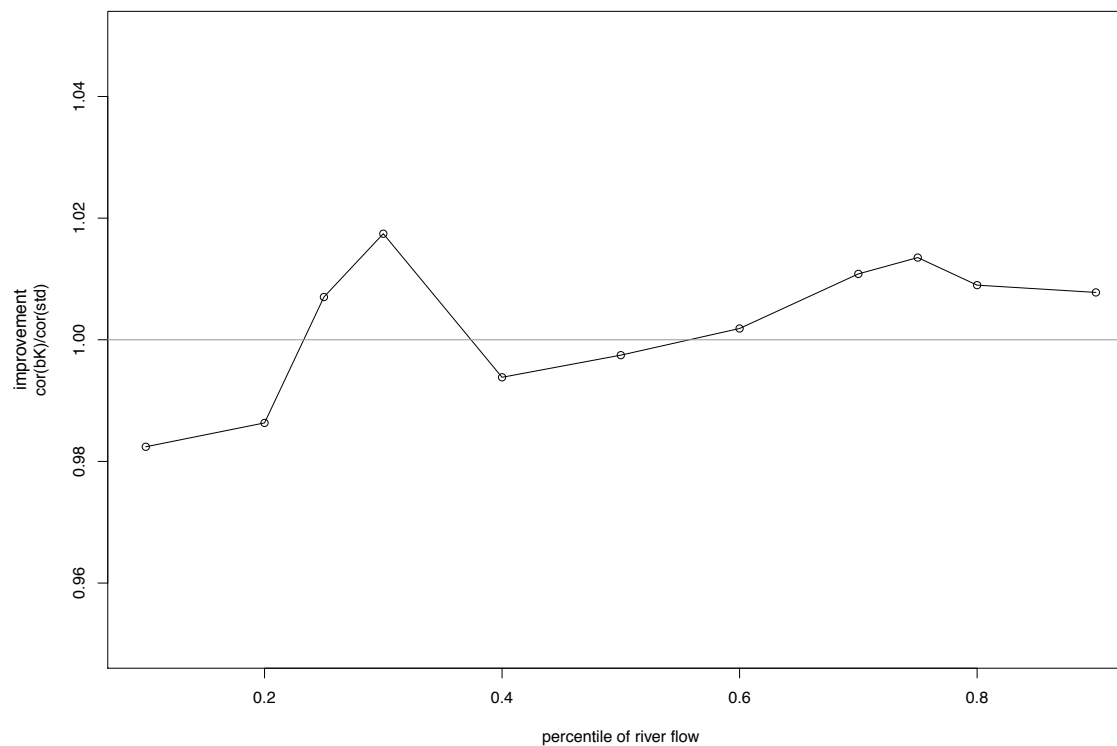
Outlooks:

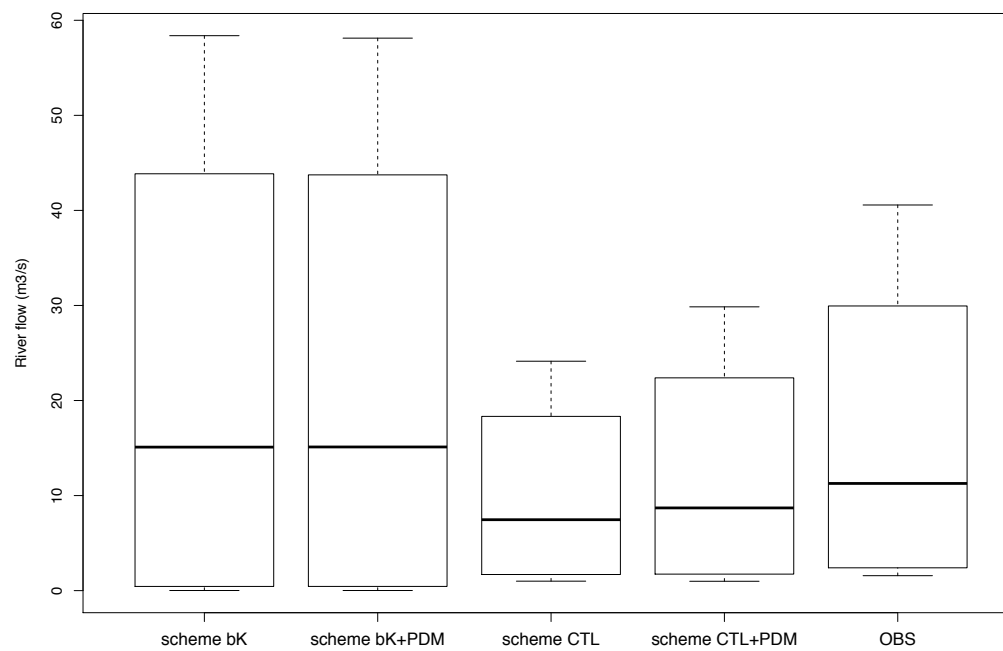
1. River flow of a UK catchment :
 - Decrease the overestimation of the variation of river flow with reducing the parameter β
 - Working with other catchment
2. Study the modelled river flow with comparison of observed flash flood events
3. Study the impact on the uncertainty of modelled precipitation on the resulting surface runoff and river flow with the new/old scheme.

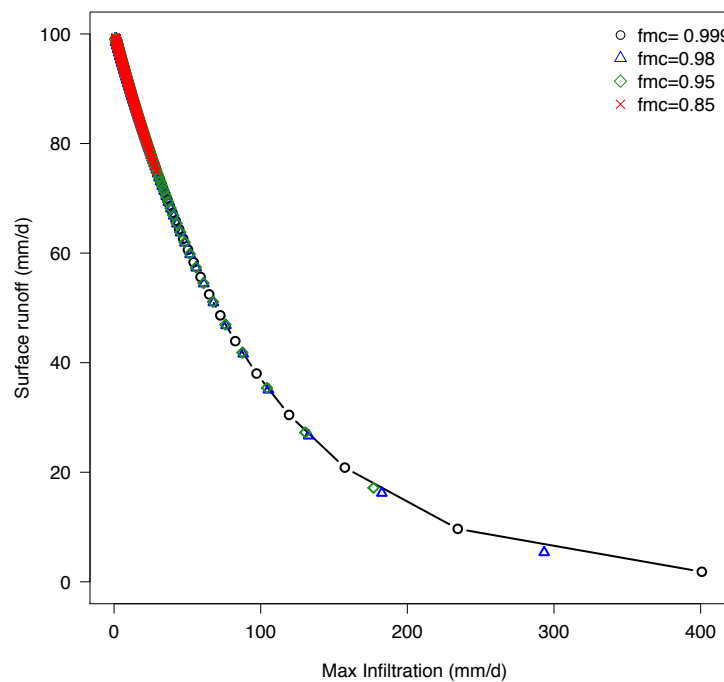
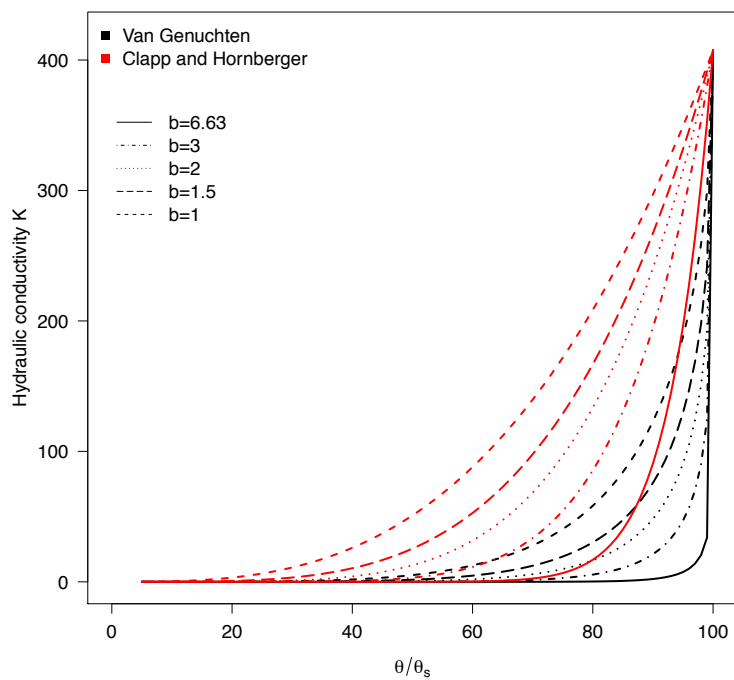
I need your help:

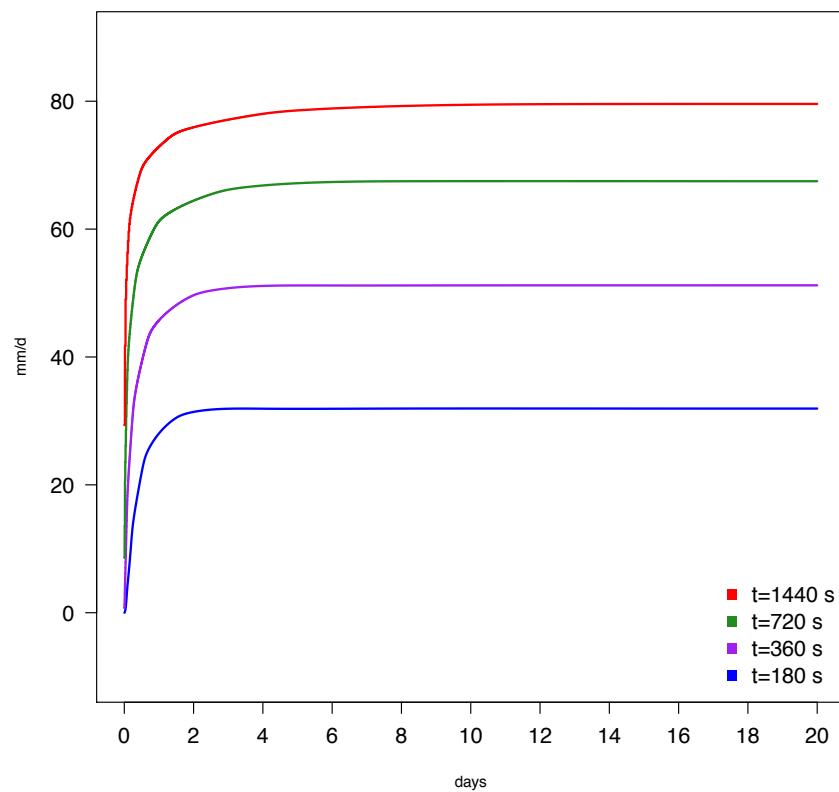
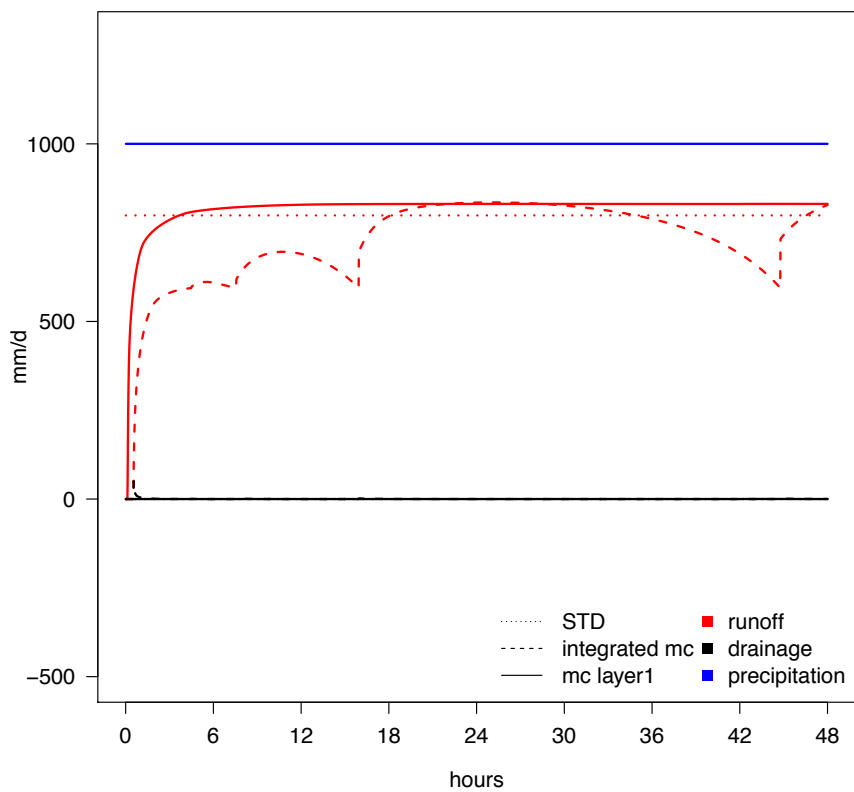
- observation data of flash floods events ?

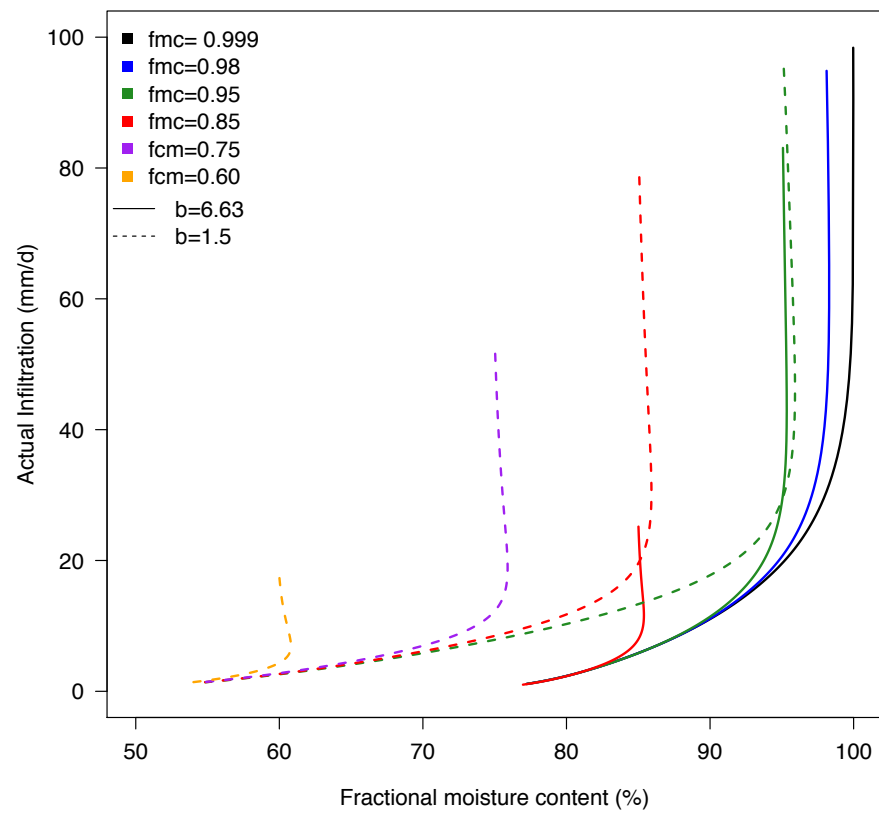
- any advice ?











Variable maximum infiltration schemes:

scheme 0: $I_{max} = \beta K_{sat}$ JULES

scheme 1: $I_{max} = \beta(W_{sat} - W)/\Delta t$ A. Mueller
(kg/m²)

scheme 2: $I_{max} = \beta(W_{sat}^{top} - W^{top})/\Delta t$

scheme 3: $I_{max} = (W_{sat} - W) + \max\left(0, W_{sat} \left[\left(1 - \frac{W}{W_{sat}}\right)^{\frac{1}{b+1}} - \left(\frac{T+M}{(b+1)W_{sat}}\right) \right]^{b+1}\right)$ CHTESSEL
(m³/m³)

scheme 4: $I_{max} = \beta K_{top}$

scheme 5: $I_{max} = K_{top} \frac{d\psi}{dz}$

New scheme (scheme bK)
Standard scheme (CTL)

Comparison of river flow in a small catchment:

Improvement

PDM deactivated (NO PDM)

PDM activated (PDM)

1991 - 2000

$$KGE_{bk} = 0.547 \quad KGE_{std} = 0.212$$

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$$\rho = 0.76 \quad \rho = 0.46$$

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$$b = 1.28 \quad b = 0.63$$

$$b = 1.28 \quad b = 0.76$$

$$a = 1.26 \quad a = 0.55$$

$$a = 1.26 \quad a = 0.69$$

Oct. 1994–1995
Peak river flow
period

$$KGE_{bk} = 0.612 \quad KGE_{std} = 0.247$$

$$KGE_{bk} = 0.607 \quad KGE_{std} = 0.523$$

$$\rho = 0.68 \quad \rho = 0.48$$

$$\rho = 0.68 \quad \rho = 0.70$$

$$b = 1.20 \quad b = 0.67$$

$$b = 1.21 \quad b = 0.78$$

$$a = 1.12 \quad a = 0.57$$

$$a = 1.12 \quad a = 0.69$$

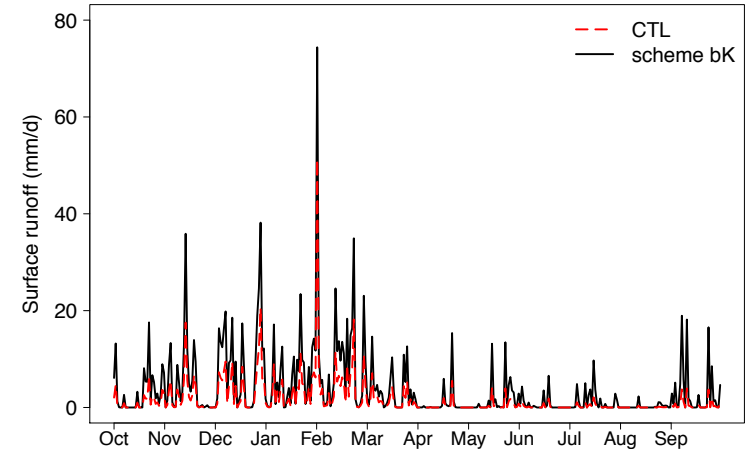
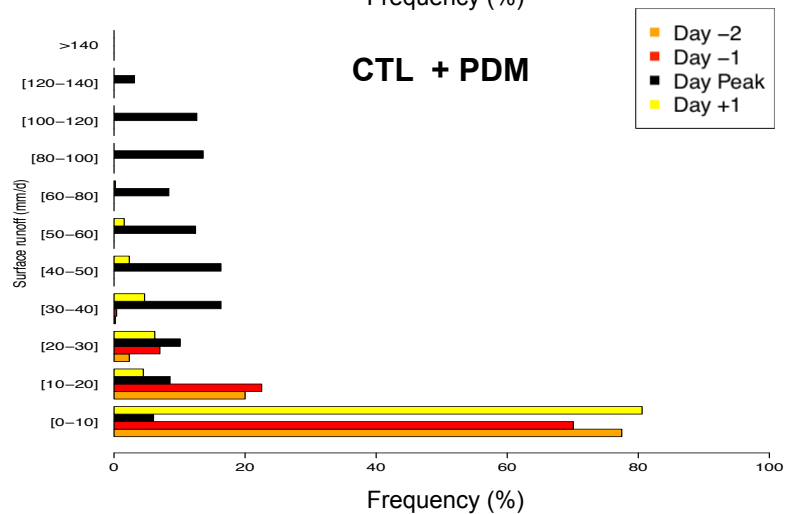
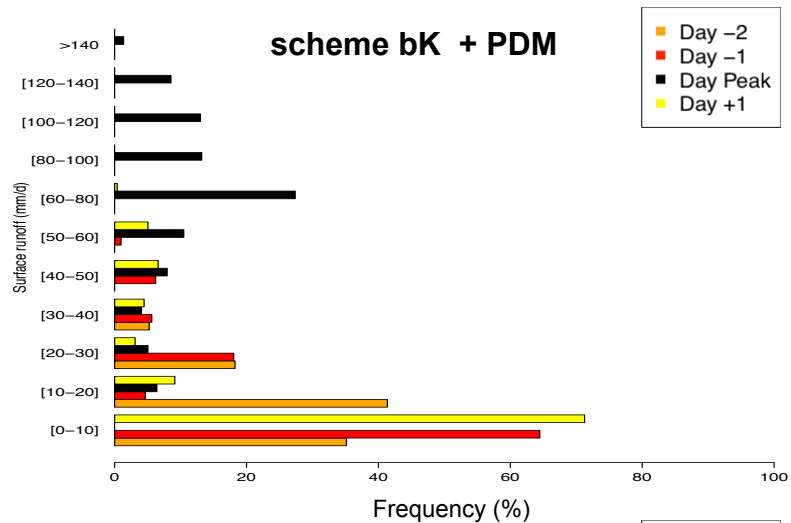
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New scheme (scheme bK)
Standard scheme (CTL)

Comparison of surface runoff in a small catchment:

Spatial frequency of surface runoff over all grid cells (515)

PDM activated (PDM)



Srunoff:

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Riverflow:

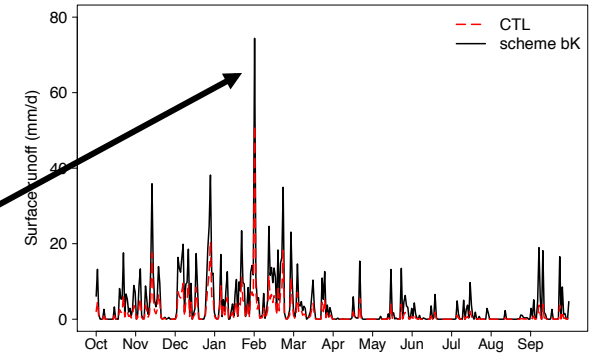
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PDM activated (PDM)

**Surface runoff
Oct. 1994–1995
Peak river flow
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Spatial frequency of surface runoff over all grid cells (515)

